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Element-free elastoplastic solid for nonsmooth multidomain dynamics

John Nordberg

john.nordberg@umu.se

UMIT Research Lab - Umeå University

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Approaches

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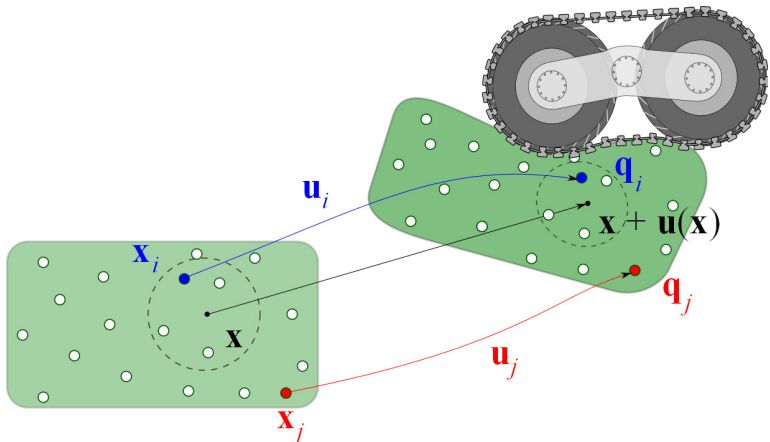
Plastic results

Solid mechanics

$$\begin{aligned}\rho \ddot{\mathbf{u}} - \nabla \cdot \boldsymbol{\sigma} &= \rho \mathbf{b} \\ \boldsymbol{\sigma} &= \mathbf{C} \boldsymbol{\epsilon}\end{aligned}$$

Multibody system dynamics

$$\begin{aligned}\mathbf{M} \ddot{\mathbf{q}} - \mathbf{G}^T \boldsymbol{\lambda} &= \mathbf{f} \\ \boldsymbol{\epsilon} \boldsymbol{\lambda} + \mathbf{g}(\mathbf{q}) &= \mathbf{0}\end{aligned}$$



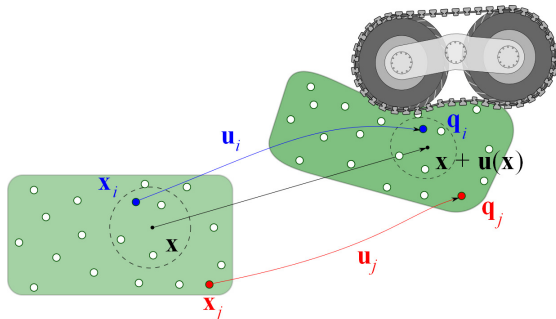
Displacement $\mathbf{u}(\mathbf{x})$

$$\mathbf{q}(\mathbf{x}) = \mathbf{x} + \mathbf{u}(\mathbf{x})$$

$$\mathbf{J} \equiv \nabla_{\mathbf{x}} \mathbf{q}(\mathbf{x}) = \mathbf{I} + \nabla_{\mathbf{x}} \mathbf{u}$$

Green-Lagrange strain tensor

$$\mathbf{E}(\mathbf{x}) = \frac{1}{2} (\mathbf{J}^T \mathbf{J} - \mathbf{I}) = \frac{1}{2} (\nabla_{\mathbf{x}} \mathbf{u} + \nabla_{\mathbf{x}}^T \mathbf{u} + \nabla_{\mathbf{x}}^T \mathbf{u} \nabla_{\mathbf{x}} \mathbf{u})$$



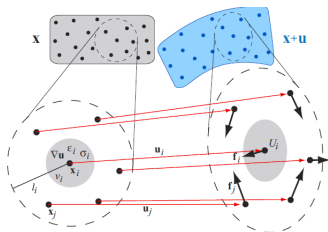
Moving least squares (MLS) approximation of the displacement field $\mathbf{u}(\mathbf{x})$

$$\mathbf{u}_\alpha(\mathbf{x}) = \sum_j^{N_p} \Psi_j(\mathbf{x}) \mathbf{u}_\alpha^j$$

$$\Psi_j(\mathbf{x}) = \mathbf{p}_\gamma(\mathbf{x}) \mathbf{A}_{\gamma\tau}^{-1}(\mathbf{x}) \mathbf{p}_\tau(\mathbf{x}^j) \mathbf{W}(\mathbf{x} - \mathbf{x}^j, h)$$

$$\mathbf{A}_{\gamma\tau}(\mathbf{x}) = \sum_j^{N_p} \mathbf{W}(\mathbf{x} - \mathbf{x}^j, h) \mathbf{p}_\gamma(\mathbf{x}^j) \mathbf{p}_\tau(\mathbf{x}^j)$$

$$\mathbf{p}(\mathbf{x}) = [1, x, y, z, yz, xz, xy, x^2, y^2, z^2]^T$$



Constraint energy

$$U = \frac{1}{2} \mathbf{g}^T \boldsymbol{\varepsilon}^{-1} \mathbf{g}$$

Constitutive relation using Voigt notation

$$\boldsymbol{\sigma} = \mathbf{C} \mathbf{E}$$

$$\mathbf{C} = \begin{bmatrix} \lambda + 2\mu & \lambda & \lambda & 0 & 0 & 0 \\ \lambda & \lambda + 2\mu & \lambda & 0 & 0 & 0 \\ \lambda & \lambda & \lambda + 2\mu & 0 & 0 & 0 \\ 0 & 0 & 0 & \mu & 0 & 0 \\ 0 & 0 & 0 & 0 & \mu & 0 \\ 0 & 0 & 0 & 0 & 0 & \mu \end{bmatrix}$$

Strain energy

$$U = \frac{1}{2} \mathbf{E} \mathbf{v}_0 \mathbf{C} \mathbf{E}$$

Elasticity strain tensor constraint and regularisation - 6D

$$\mathbf{g}^i(\mathbf{q}) = \mathbf{E}^i \quad \boldsymbol{\varepsilon} = (\mathbf{v}_0 \mathbf{C})^{-1}$$

Jacobian where $\mathbf{K}_{\tau\eta} \equiv \nabla_{\tau} \mathbf{u}_{\eta}$

$$\mathbf{G}_{\alpha\beta}^i(\mathbf{q}) = \frac{\partial g_{\alpha}^i}{\partial q_{\beta}} = \frac{\partial g_{\alpha}^i}{\partial E_{\gamma}} \frac{\partial E_{\gamma}}{\partial \mathbf{K}_{\tau\eta}} \frac{\partial \mathbf{K}_{\tau\eta}}{\partial q_{\beta}}$$



Multibody dynamics - numerical solver

Linearized variational time stepper SPOOK (Lacoursière [2, 3])

$$q_{n+1} = q_n + h\dot{q}_{n+1}$$

$$\underbrace{\begin{bmatrix} M & -G^T & -\bar{G}^T \\ G & \Sigma & 0 \\ \bar{G} & 0 & \bar{\Sigma} \end{bmatrix}}_H \underbrace{\begin{pmatrix} \dot{q}_{n+1} \\ \lambda \\ \bar{\lambda} \end{pmatrix}}_z = \underbrace{\begin{pmatrix} M\dot{q}_n + hf_n \\ -\frac{4}{h}\gamma g + \gamma G\dot{q}_n \\ \omega_n \end{pmatrix}}_{-r}$$

regularization and stabilization matrices

$$\Sigma = \frac{4}{h^2} \text{diag} \left(\frac{\varepsilon_1}{1 + 4\frac{\tau_1}{h}}, \frac{\varepsilon_2}{1 + 4\frac{\tau_2}{h}}, \dots \right) \quad \varepsilon = (v_0 C)^{-1}$$

$$\bar{\Sigma} = \frac{1}{h} \text{diag} (\gamma_1, \gamma_2, \dots)$$

$$\gamma = \text{diag} \left(\frac{1}{1 + 4\frac{\tau_1}{h}}, \frac{1}{1 + 4\frac{\tau_2}{h}}, \dots \right)$$



Nonsmooth MBD - numerical solver

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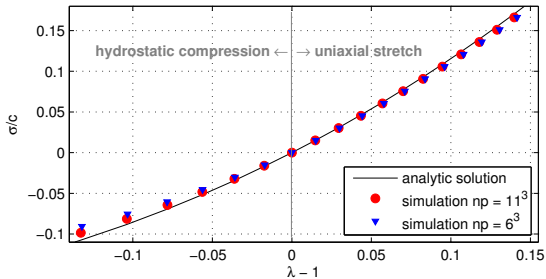
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Including frictional contacts, impacts, joint and motor limits lead to limits and complementarity conditions on the solution variables

$$\begin{aligned} Hz + r &= w_+ - w_- \\ 0 &\leq w_+ \perp z - l \geq 0 \\ 0 &\leq w_- \perp u - z \geq 0 \end{aligned}$$

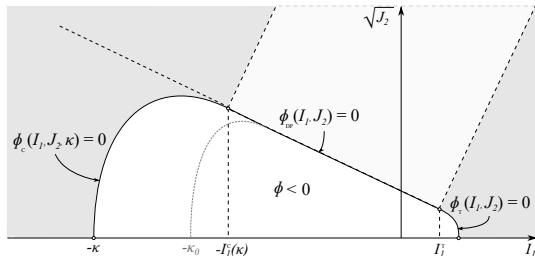
The problem transforms from linear system to a mixed linear complementarity problem (MLCP)



Elastoplastic terrain model

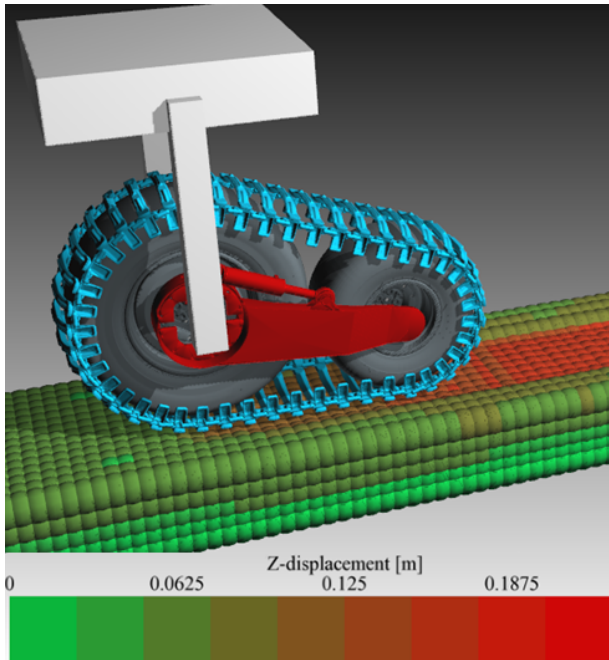
Elastic and plastic strain components $\mathbf{E} = \mathbf{E}^e + \mathbf{E}^p$

Plastic flow rule $d\mathbf{E}^p = d\lambda^p \frac{\partial \Phi}{\partial \boldsymbol{\sigma}}$ when yield $\Phi(\boldsymbol{\sigma}) > 0$



Capped Drucker-Prager plasticity model (Dolarevic [1])

$$\Phi(\boldsymbol{\sigma}, \kappa) = \begin{cases} \Phi_t(I_1, J_2) & I_1 \geq I_1^t \\ \Phi_e(I_1, J_2) & I_1^t \leq I_1 \leq I_1^c(\kappa) \\ \Phi_c(I_1, J_2, \kappa(\text{tr } \mathbf{E}^p)) & I_1 \leq I_1^c(\kappa) \end{cases} \quad (1)$$





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- [1] S. Dolarevic et al. A modified three-surface elasto-plastic cap model and its numerical implementation. *Comput. Struct.*, 85(7-8):419-430, April 2007.
- [2] C. Lacoursière, M. Linde, SPOOK: a variational time-stepping scheme for rigid multibody systems subject to dry frictional contacts, submitted (2013).
- [3] C. Lacoursière, Ghosts and Machines: Regularized Variational Methods for Interactive Simulations of Multibodies with Dry Frictional Contacts, PhD thesis, UmeåUniversity, Sweden, (2007)

Elastoplastic terrain model

Drucker-Prager for cohesive soil

$$\Phi_e (I_1, J_2) = \sqrt{J_2} + \eta (\phi) \frac{I_1}{3} - \xi (\phi) c \quad (2)$$

Tension cap

$$\Phi_t (I_1, J_2) = (I_1 - T + R_t)^2 + J_2 - R_t^2 \quad (3)$$

Compression cap

$$\Phi_c (I_1, J_2, \kappa (\text{tr } \epsilon^p)) = \frac{(I_1 - a (\kappa))^2}{R^2} + J_2 - b (\kappa)^2 \quad (4)$$

Cap variables - for compressive hardening

$$\kappa = \kappa_0 + \frac{1}{D} \ln \left(1 + \frac{\text{tr}(\epsilon^p)}{W} \right) \quad (5)$$

where κ_0 is the initial position of the compression cap (Dolarevic [1]).